Strength and Durability of Concretes with Slag-Fly Ash-Portland Cement

A. Borsoi, S. Collepardi, L. Coppola, R. Troli and M. Collepardi

Synopsis: Composite cements containing portland cement (50%), fly ash (25%) and slag (25%), all interground at a Blaine fineness of about 400 or 500 m²/kg, were produced.

Superplasticized concretes with a slump of 200-230 mm were manufactured by using sulfonated naphthalene (SN) or acrylic polymer (AP). The dosage of superplasticizers was a little higher (10% more) when finer cements were used. The AP superplasticizer was more effective than that based on SN in terms of lower dosage (20% less) and lower water-cementitious material ratio (10% less) at equal workability. Consequently, higher compressive strength were obtained for concretes with the AP admixture rather than with the NS superplasticizer.

The better performance of the AP superplasticizer with respect to the SN admixture was independent of the curing temperature (5° or 20°) at early (1 day) and later ages (28-90 days).

All the concrete mixtures perform very well for the durability behavior in terms of lower CO_2 penetration and chloride diffusion. However, due to the lower water-cementitious material ratio (0.29 vs. 0.32) concretes with the acrylic polymer are potentially more durable than those with the naphthalene-based superplasticizer.

<u>Keywords:</u> Carbonation, Chloride Penetration, Composite Cement, Compressive Strength, Fly Ash, Slump, Slump Loss, Slag Superplasticizer.

Antonio Borsoi is a laboratory technician of Enco. He is active in the area of concrete mixture design. He is author of several papers in the field of superplasticized concrete mixtures.

Silvia Collepardi is a research civil engineer of Enco, Spresiano, Italy. She is working in the field of concrete durability and superplasticized concrete mixtures and has published several papers in this area.

Luigi Coppola is a research civil engineer and technical director of Enco, Spresiano, Italy. He has authored numerous papers on various aspects of concrete technology, durability and mix-design. He was acknowledged by a CANMET-ACI award for his contribution to the fundamental knowledge of concrete durability.

Roberto Troli is a research civil engineer and director of the Enco laboratory. He is author of several papers in the field of concrete technology, particularly in the area of chemical and mineral admixtures as well as in the durability of concrete.

Mario Collepardi is Professor of Materials Science and Technology at the Politecnico di Milano, Milan, Italy. He is author or co-author of numerous papers on concrete technology and cement chemistry. He is also the recipient of several awards for his contributions to the fundamental knowledge of superplasticizers and their use in concrete.

INTRODUCTION

Composite cements, based on ternary blends of portland cement, fly ash, and ground granulated blast-furnace slag, may be produced, in agreement with the European Standard EN 197/1(1), with a slag + fly ash content up to 60% (CEM V/A) or 80% by mass (CEM V/B). However, in Europe composite cements are not widely used in practice.

Previous works on ternary blends of portland cement, slag, and fly ash were studied by Berry (2), Swamy (3), Douglas and Pouskouleli (4), Nagataki (5), Dehuai and Zhaoyuan (6) and Collepardi et al. (7).

The main purpose of the present paper was to study the influence of different superplasticizers on concrete mixtures manufactured by using fly ash-slag-portland cement with a total portland cement replacement of 50% by mass. The following properties were examined: workability, slump-loss, compressive strength, and durability of concrete mixtures, all manufactured with a water-cementitious material ratio (w/cm) as low as about 0.3.

MATERIALS

Cementitious materials. The chemical analysis of the ingredients (portland cement, fly ash, and slag) used to manufacture blended cements are shown in Table 1.

Table 2 shows the composition of four composite cements (A, B, C, and D) all with 50% portland cement replaced by 25% of fly ash and 25% of ground granulated blast furnace slag, the main difference being the fineness of the three ingredients. Cements A and C were manufactured by blending unground fly ash with portland cement and slag ground at the same fineness (about 400 or 500 m²/kg). Cements B and D were manufactured by blending ground fly ash with the other two ingredients at the same fineness (about 400 or 500 m²/kg). In a previous paper (7) more details can be found for setting time and compressive strength of the four blended cements according to the EN 197/1 standard.

Aggregates. Natural sand (fineness modulus of 3.09) and two coarse aggregates were used. For all concrete mixtures a combined aggregate was used with 25% of sand, 40% of gravel (4-16 mm), and 35% of coarse aggregate (10-25 mm).

Superplasticizers. A 40% aqueous solution of sulfonated naphthalene-based (SN) superplasticizer or a 30% of acrylic polymer (AP) aqueous solution were used. Both superplasticizers came from Mapei, Milan (Italy).

Concrete mixtures. Table 3 shows the composition of concrete mixtures. The superplasticizer dosage (in the range of 0.8-1.1% by mass of cementitious material) was adjusted to obtain approximately the same slump level (200-230 mm) at about the same w/cm of 0.3.

The content of the cementitious material (including fly ash and slag) was about 470 kg/m^3 which corresponds to a content of portland cement of about 235 kg/m^3 .

METHODS

The following properties were determined:

- slump just after mixing (5 min) and after some time of agitation at room temperature (slump loss);
- compressive strength of 100 mm-cube concrete specimens cured at 5°C or 20°C;
- depth of carbonation as measured by the phenolphthalein test (RILEM CPC 18) of concrete specimens exposed to air after demoulding at 1 day;

• chloride penetration through the AgNO₃ and fluorescein test (8) of concrete specimens exposed to a 10% NaCl aqueous solution after a wet curing of 1 week and an air curing of 3 weeks.

RESULTS

Workability. Table 3 indicates that, at a given slump level, the *AP* admixture was more effective than the *SN*-based superplasticizers in terms of lower *w/cm* (10% less) of the concrete mixtures and lower superplasticizer dosage (20% less).

The increase in the specific surface area of the composite cement causes a negligible increase of the dosage of SN- or AP-based superplasticizer unless all the ingredients are ground at a Blaine fineness of about 500 m²/kg as in the cement D for mixtures No. 4 and No. 8 of Table 3.

Slump-loss. Figure 1 shows the slump-loss curves of concrete mixtures with different superplasticizer type (AP vs. SN) or composite cements characterized by different fineness. For a given composite cement, the slump loss is much lower in concrete mixtures with the AP-based superplasticizer than in the corresponding mixture with the SN-based admixture.

On the other hand, for a given superplasticizer type (SN or AP), the slumploss is mitigated by the use of cements with lower specific surface area (Fig.1).

Compressive strength Figures 2, 3, 4 and 5 show the cube compressive strength results of concrete cured at 5°C and 20°C at R.H. of 95% for mixtures containing cements *A*, *B*, *C* and *D* respectively. In general, the compressive strength at 20°C is higher than that at 5°C at both early and later ages. However, even concrete specimens cured at lower temperature (5°C) reach strength levels as high as 35-50 MPa within 3 days. Although the portland cement content is as low as 235 kg/m³, the strength level of all these concretes is relatively high at longer ages: 65-80 MPa at 28 days and 70-90 MPa at 90 days. This performance is related to the action of the cementitious activity of fly ash and slag combined with a *w/cm* as low as 0.3 due to the presence of superplasticizer.

The compressive strength of concrete with the *AP*-based superplasticizer is always higher (about 10% more) than that of the corresponding concrete with the *SN*-based admixture. However, the difference also depends on the specific surface area of the individual ingredients of the composite cement

Carbonation. Figure 6 shows the carbonation depth (x) as a function of time (t). After about 1 year of exposure to air, the carbonation depth is un-detectable or negligible in all the eight studied concretes. Then, that carbonation does not pose problems for corrosion of the metallic reinforcements due to very low permeability of these concretes containing high volumes of fly ash and ground slag.

Due, to the lower w/cm of concretes with the AP-based admixtures the carbonation rate is definitely lower with respect to that of the corresponding concretes with the SN-based superplasticizer.

The difference in the carbonation rate between concretes with cements characterized by different specific surface area is negligible.

Chloride penetration Figure 7 shows the chloride penetration depth with time for the mixtures with composite cements.

Chloride penetration does not seem to depend on the fineness of the cementitious material of the composite cement.

Again, the chloride penetration rate is definitely lower in concretes with AP-based superplasticizer, with respect to those with the SN-based admixture, for the lower w/cm (0.29 vs. 0.32)

CONCLUSIONS

Superplasticized concretes with composite cements (25% fly ash, 25% ground slag, a 50% portland cement) perform very well in concrete mixtures in terms of good workability (190-220 mm), high compressive strength (65-80 MPa at 28 days), and excellent durability behavior (negligible carbonation and very low chloride penetration).

The compressive strength is slightly increased by the grinding action of both fly ash and slag.

The durability behavior does not seem to be significantly improved by increasing the fineness of the cementitious materials.

The superplasticizer based on acrylic polymer appears to be more effective than that based on sulfonated naphthalene in terms of lower *w/cm*, lower slumploss, higher compressive strength and better durability behavior for the lower carbonation rate and chloride penetration.

ACKNOWLEDGEMENT

The authors are very grateful to Cementirossi, Piacenza (Italy), for grinding all the cementitious materials for blended cement.

REFERENCES

1. European Standard EN 197-1, "Cement Composition, Specification and Conformity Criteria", Part 1, Common Cements, 1992.

- 2. Berry, E.E., "Strength Development of Some Blended-Cement Mortars", *Cement and Concrete Research*, V. 10, pp. 1-11, 1980.
- 3. Swamy, R.N., "Cement Replacement Materials", Survey University Press, 1986.
- 4. Douglas, E., and Pouskouleli, G., "Prediction of Compressive Strength of Mortars Made with Portland Cement–Blast Furnace Slag-Fly Ash Blends", *Cement and Concrete Research*, V. 21, pp. 523-534, 1991.
- 5. Nagataki, S., "Mineral Admixtures in Concrete: State of the Art and Trends", ACI SP-144, pp. 473-484, 1994.
- 6. Dehuai, W., and Zhaouyan, C., "On Predicting Compressive Strength of Mortars with Ternary Blends of Cement, GGBFS and Fly Ash", *Cement and Concrete Research*, V. 27, pp. 487-493, 1997.
- 7. Collepardi, S., Corinaldesi, G., Moriconi, G., Bonora, G., and Collepardi, M., "Durability of High Performance Concretes with Pozzolanic and Composite Cements", *Proceedings of CANMET-ACI Conference on Durability of Concrete*, Ed. V.M. Malhotra, Barcelona, Spain, 1999.
- 8. Collepardi, M., Marcialis, A., and Turriziani, R., "Penetration of Chloride Ions into Cement Pastes and Concretes", *Journal of American Ceramic Society*, V. 53, pp. 534-535, 1972.

Table 1 – Chemical composition of portland cement, fly ash, and slag used to produce blended cements.

Oxyde (%)	Portland cement	Fly Ash	Slag	
SiO ₂	21.25	59.94	36.50	
Al_2O_3	4.33	22.87	11.67	
Fe ₂ O ₃	1.85	4.67	1.01*	
TiO ₂	0.13	0.94	0.20	
CaO	64.30	3.08	38.95	
MgO	1.81	1.55	8.08	
SO_3	3.70	0.35	1.00**	
K ₂ O	0.71	2.19	0.42	
Na ₂ O	0.17	0.62	0.34	
l.o.i.	1.50	3.34	1.28	

*as FeO **as S

Table 2 - Composition and Blaine fineness of composite cements CEM V/A

			Portland		Fly Ash			Ground Slag	
Blended Cement		Cement Fineness (m ² /kg)		Unground Ground Ground					
				Fineness (m ² /kg)			Fineness (m ² /kg)		
Туре	Fineness (m ² /g)	395	504	351	395	482	412	517	
A	384	50%	-	25%	-	-	25%	-	
В	392	50%	-	-	25%	-	25%	-	
C	472	-	50%	25%		-		25%	
D	499	-	50%	-	-	25%	-	25%	

Table 3 – Composition of concrete mixtures.

Mix	Total Cement Content		Gravel 10-25 mm	Gravel 4-16 mm	Sand 0-4 mm	Water	Super- plasticizer	w/cm	Slump
No.	Туре	(kg/m ³)	(kg/m ³)	(kg/m ³)	(kg/m ³)	(kg/m ³)	(type%cem)		(mm)
1	A	469	459	738	648	150	SN-1.00	0.32	220
2	В	471	461	741	651	150	SN-1.00	0.32	220
3	С	467	457	735	646	149	SN-1.05	0.32	200
4	D	472	462	744	653	151	SN-1.10	0.32	200
5	A	470	460	741	651	135	AP-0.80	0.29	230
6	В	473	463	745	655	135	AP-0.80	0.29	220
7	С	466	458	737	648	134	AP-0.80	0.29	210
8	D	474	464	746	656	136	AP-0.90	0.29	210

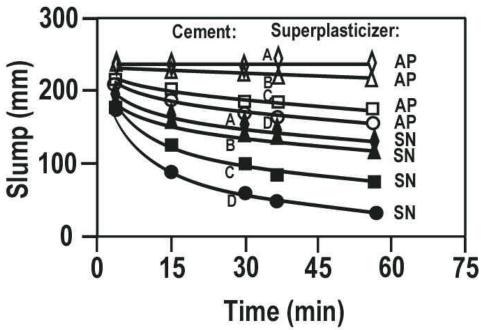


Fig. 1 - Slump-loss at 20°C of concrete mixtures with AP or SN superplasticizers (cements A, B, C and D in Table 2)

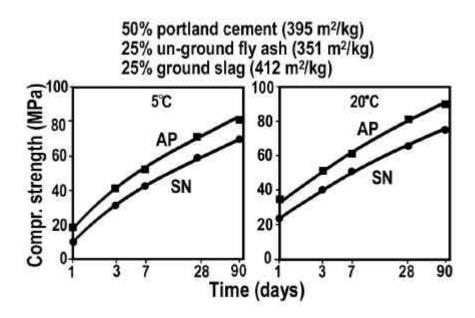


Fig. 2 - Influence of temperature and superplasticizer type on compressive strength versus time (cement A).

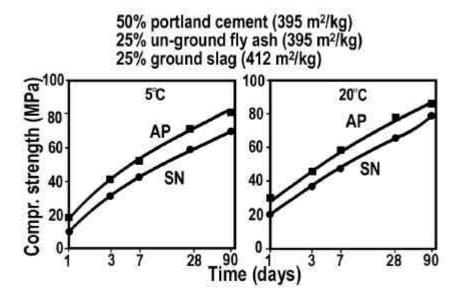


Fig. 3 - Influence of temperature and superplasticizer type on compressive strength versus time (cement B).

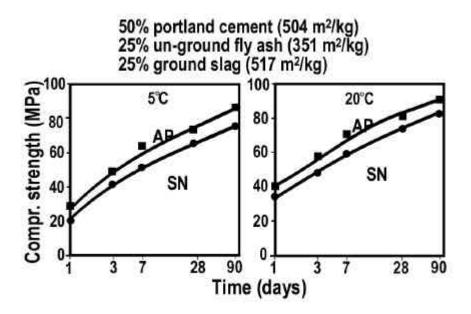


Fig. 4 - Influence of temperature and superplasticizer type on compressive strength versus time (cement C).

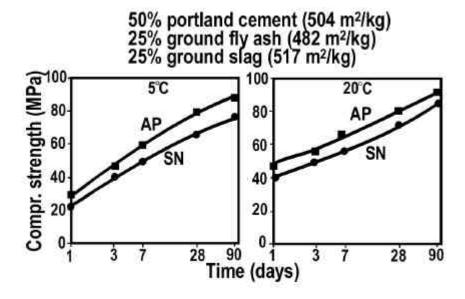


Fig. 5 - Influence of temperature and superplasticizer type on compressive strength versus time (cement D).

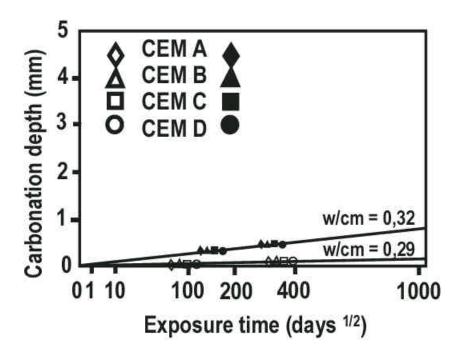


Fig. 6 - Carbonation depth in concrete mixtures with different w/cm.

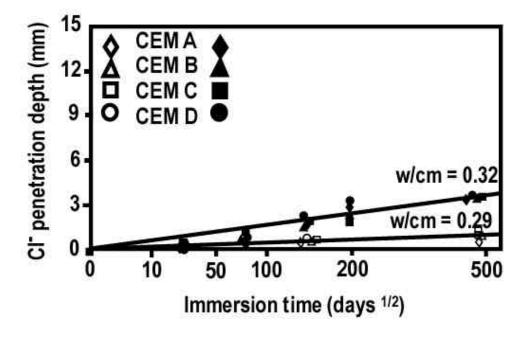


Fig. 7 - Chloride penetration in concrete mixtures with different w/cm